### PRELIMINARY HYDROLOGY REPORT

For

## **Oakmont – Redlands at Nance Industrial**

### 22-05040

### **PROJECT LOCATION**

255 East Nance Street, Perris, CA

### DEVELOPER

Oakmont Industrial Group 3520 Piedmont Ave Suite 100 Atlanta, GA 30305 (949) 215-3796

### PREPARED BY

Huitt-Zollars, Inc. 3990 Concours Suite 330 Ontario, CA 91764 Ph: (909) 941-7799 Fax: (909) 941-7789

#### **PREPARATION DATE**

May 17, 2022 Revised: TBD

### **HZ PROJECT NUMBER**

R314482.01

) White

David White, P.E. C52921, Exp 12/31/2022



# **Table of Contents**

Title	Page 1
Introduction	1
Purpose	1
Existing Condition	1
Proposed Condition	1
Hydrologic Analysis	2
Results	2

# List of Appendices

Appendix A	Preliminary Hydrology Maps
Appendix B	100-year Rational Method
Appendix C	Hydrologic Soils Group and Isohyetal Maps

### Introduction

This preliminary 100-year hydrologic analysis is prepared for Oakmont Industrial Group. The objective of this project is to build an industrial warehouse facility located at the southeast corner of Las Palmas and Nance Street in Perris, CA. The proposed building is approximately 202,500 square feet in size on approximately 9.57 acres of partially developed land. In order to treat stormwater runoff, the project's stormwater management strategy incorporates a bio-retention basin.

### Purpose

The purpose of this report is to present the drainage concept and design flow rates for the project site. The existing and proposed hydrology maps reflect the tributary drainage areas as well as the 100-year storm runoff flow rates.

### **Existing Condition**

The project site consists of a rectangular shaped parcel which is partially developed with residential buildings and industrial yards.

The site generally slopes 0.4% from the northern to the southern edge of the property.

The site's maximum  $1456.3\pm$  feet mean sea level (MSL) elevation is located at the northwest corner of the parcel. The site's minimum  $1453.8\pm$  feet MSL elevation is located at the southwest corner of the parcel. Runoff sheet flows south to the southern property line, goes through openings in the screen wall, and eventually discharges to the Markham Street storm drain. The existing storm drain in Markham Street conveys the runoff to the San Jacinto River Basin.

From the hydrologic analysis, the existing 100-year stormwater runoff is approximately 15.32 cfs.

### **Proposed Condition**

Under the proposed condition, the site's runoff will be directed to an on-site bio-retention basin located along the eastern portion of the property. See Appendix A for proposed on-site hydrology map. The captured water will drain into an on-site storm drain line which will convey the runoff to the proposed flood control master planned storm drain (Line D-2) west of the property.

Runoff from the site will sheet flow and surface drain into the bio-retention basin.

### Hydrologic Analysis

A hydrologic analysis was prepared using the methodology outlined in the Riverside County Flood Control District (RCFCD) Hydrology Manual. A rational method analysis was completed for the proposed 100-year return event using Civild software.

The 100-year  $(Q_{100})$  1-hour rainfall depth was taken from the isohyetal maps in the County of Riverside Hydrology Manual. The hydrologic soils type for the site is "C" and was taken from the soil map in the Hydrology Manual (see Appendix E for reference maps). A "commercial" land use was used with an AMC of III for the 100-year storm event.

### Results

The bio-retention basin has been sized for water quality purposes. The proposed development resulted in a  $Q_{100}$  peak flow of approximately 18.88 cfs. However, we are discharging to a master planned storm drain (Line D-2) which is sized for the 100-year developed runoff.

All proposed drainage and storm drain facilities will be sized adequately for  $Q_{100}$  peak flow. Additional calculations will be provided in the final report including storm drain hydraulics and catch basin sizing.

Appendix A Preliminary Hydrology Maps





**CITY OF PERRIS EXISTING CONDITION** HYDROLOGY MAP SHEET OAKMONT - NANCE STREET WAREHOUSE 1 OF 1 255 NANCE STREET

LEGEND HYDROLOGY MODEL NODE NUMBER NO.  $\frac{\mathbf{A-11}}{\mathbf{7.40 Ac}}$ TRIBUTARY AREA IN ACRES 673'-\_\_\_\_LENGTH OF FLOW DRAINAGE BOUNDARY **— · · —** FLOW LINE \_\_\_\_\_ · · · \_\_\_\_\_ FLOW DIRECTION

**OWNER/DEVELOPER** OAKMONT INDUSTRIAL GROUP 3520 PIEDMONT AVE SUITE 100 ATLANTA, GA 30305 PHONE (949) 215–3796 CONTACT PERSON: JOHN ATWELL

CIVIL ENGINEER HUITT-ZOLLARS, INC. 3990 CONCOURS, SUITE 330 ONTARIO, CALIFORNIA 91764 PHONE: (909) 941-7799 CONTACT: DAVID WHITE

 $\mathbb{N}$ 

SCALE 1" = 30'



	-	_
д.,	<u> </u>	
	Š	
,	3	
-		
		7
		-2

NO. HYDROLOGY MODEL NODE NUMBER

TRIBUTARY AREA IN ACRES DRAINAGE BOUNDARY FLOW LINE FLOW DIRECTION DRAINAGE INLET/CATCH BASIN (CB)

SHEET 1 OF 1

Appendix B 100-year Rational Method

```
Riverside County Rational Hydrology Program
CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2005 Version 7.1
   Rational Hydrology Study Date: 05/18/22 File:4482Q100E.out
                               _____
OAKMONT - REDLANDS AT NANCE INDUSTRIAL
100 YEAR STORM - EXISTING RATIONAL METHOD
44820100E
AC
_____
******** Hydrology Study Control Information *********
English (in-lb) Units used in input data file
_____
Program License Serial Number 6145
 _____
Rational Method Hydrology Program based on
Riverside County Flood Control & Water Conservation District
1978 hydrology manual
Storm event (year) = 100.00 Antecedent Moisture Condition = 3
Standard intensity-duration curves data (Plate D-4.1)
For the [ Perris Valley ] area used.
10 year storm 10 minute intensity = 1.880(In/Hr)
10 year storm 60 minute intensity = 0.780(In/Hr)
100 year storm 10 minute intensity = 2.690(In/Hr)
100 year storm 60 minute intensity = 1.120(In/Hr)
Storm event year = 100.0
Calculated rainfall intensity data:
1 hour intensity = 1.120(In/Hr)
Slope of intensity duration curve = 0.4900
Process from Point/Station 1.000 to Point/Station
                                                    2.000
**** INITIAL AREA EVALUATION ****
Initial area flow distance = 630.000(Ft.)
Top (of initial area) elevation = 1456.200(Ft.)
Bottom (of initial area) elevation = 1453.800(Ft.)
Difference in elevation = 2.400(Ft.)
Slope = 0.00381 s(percent) = 0.38
TC = k(0.480) * [(length^3) / (elevation change)]^{0.2}
Initial area time of concentration = 19.267 min.
Rainfall intensity = 1.954(In/Hr) for a 100.0 year storm
SINGLE FAMILY (1 Acre Lot)
Runoff Coefficient = 0.819
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 3) = 84.40
Pervious area fraction = 0.800; Impervious fraction = 0.200
```

Initial subarea runoff = 15.323(CFS)
Total initial stream area = 9.570(Ac.)
Pervious area fraction = 0.800
End of computations, total study area = 9.57 (Ac.)
The following figures may
be used for a unit hydrograph study of the same area.
Area averaged pervious area fraction(Ap) = 0.800
Area averaged RI index number = 69.0

Riverside County Rational Hydrology Program CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2005 Version 7.1 Rational Hydrology Study Date: 05/18/22 File:4482Q100POFF.out ------\_\_\_\_\_ OAKMONT - REDLANDS AT NANCE INDUSTRIAL 100 YEAR STORM - PROPOSED RATIONAL METHOD OFFSITE 44820100POFF AC \_\_\_\_\_ \*\*\*\*\*\*\*\* Hydrology Study Control Information \*\*\*\*\*\*\*\*\* English (in-lb) Units used in input data file \_\_\_\_\_ Program License Serial Number 6145 \_\_\_\_\_ Rational Method Hydrology Program based on Riverside County Flood Control & Water Conservation District 1978 hydrology manual Storm event (year) = 100.00 Antecedent Moisture Condition = 3 Standard intensity-duration curves data (Plate D-4.1) For the [ Perris Valley ] area used. 10 year storm 10 minute intensity = 1.880(In/Hr) 10 year storm 60 minute intensity = 0.780(In/Hr) 100 year storm 10 minute intensity = 2.690(In/Hr) 100 year storm 60 minute intensity = 1.120(In/Hr) Storm event year = 100.0Calculated rainfall intensity data: 1 hour intensity = 1.120(In/Hr)Slope of intensity duration curve = 0.4900Process from Point/Station 61.000 to Point/Station 62.000 \*\*\*\* INITIAL AREA EVALUATION \*\*\*\* Initial area flow distance = 656.000(Ft.) Top (of initial area) elevation = 1456.510(Ft.) Bottom (of initial area) elevation = 1454.990(Ft.) Difference in elevation = 1.520(Ft.) Slope = 0.00232 s(percent) = 0.23  $TC = k(0.300) * [(length^3) / (elevation change)]^{0.2}$ Initial area time of concentration = 13.517 min. 2.325(In/Hr) for a 100.0 year storm Rainfall intensity = COMMERCIAL subarea type Runoff Coefficient = 0.891 Decimal fraction soil group A = 0.000Decimal fraction soil group B = 0.000Decimal fraction soil group C = 1.000Decimal fraction soil group D = 0.000RI index for soil (AMC 3) = 84.40Pervious area fraction = 0.100; Impervious fraction = 0.900

Initial subarea runoff = 1.243(CFS)
Total initial stream area = 0.600(Ac.)
Pervious area fraction = 0.100
End of computations, total study area = 0.60 (Ac.)
The following figures may
be used for a unit hydrograph study of the same area.
Area averaged pervious area fraction(Ap) = 0.100
Area averaged RI index number = 69.0

```
Riverside County Rational Hydrology Program
CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2005 Version 7.1
    Rational Hydrology Study Date: 03/29/22 File:4482Q100P.out
                                ------
   _____
OAKMONT - REDLANDS AT NANCE INDUSTRIAL
100 YEAR STORM - PROPOSED RATIONAL METHOD
44820100P
AC
_____
******** Hydrology Study Control Information *********
English (in-lb) Units used in input data file
_____
Program License Serial Number 6145
 _____
Rational Method Hydrology Program based on
Riverside County Flood Control & Water Conservation District
1978 hydrology manual
Storm event (year) = 100.00 Antecedent Moisture Condition = 3
Standard intensity-duration curves data (Plate D-4.1)
For the [ Perris Valley ] area used.
10 year storm 10 minute intensity = 1.880(In/Hr)
10 year storm 60 minute intensity = 0.780(In/Hr)
100 year storm 10 minute intensity = 2.690(In/Hr)
100 year storm 60 minute intensity = 1.120(In/Hr)
Storm event year = 100.0
Calculated rainfall intensity data:
1 hour intensity = 1.120(In/Hr)
Slope of intensity duration curve = 0.4900
Process from Point/Station 1.000 to Point/Station
                                                    2.000
**** INITIAL AREA EVALUATION ****
Initial area flow distance = 1000.000(Ft.)
Top (of initial area) elevation = 1457.960(Ft.)
Bottom (of initial area) elevation = 1453.260(Ft.)
Difference in elevation = 4.700(Ft.)
Slope = 0.00470 s(percent) = 0.47
TC = k(0.300) * [(length^3) / (elevation change)]^{0.2}
Initial area time of concentration = 13.890 min.
                     2.294(In/Hr) for a 100.0 year storm
Rainfall intensity =
COMMERCIAL subarea type
Runoff Coefficient = 0.891
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 3) = 84.40
Pervious area fraction = 0.100; Impervious fraction = 0.900
```

```
Initial subarea runoff = 7.933(CFS)
Total initial stream area = 3.880
                                 3.880 (Ac.)
Pervious area fraction = 0.100
Process from Point/Station 2.000 to Point/Station
                                                            3.000
**** STREET FLOW TRAVEL TIME + SUBAREA FLOW ADDITION ****
Top of street segment elevation = 1453.260(Ft.)
End of street segment elevation = 1451.450(Ft.)
Length of street segment = 273.000(Ft.)
Height of curb above gutter flowline =
                                         6.0(In.)
Width of half street (curb to crown) = 20.000(Ft.)
Distance from crown to crossfall grade break = 18.500(Ft.)
Slope from gutter to grade break (v/hz) = 0.020
Slope from grade break to crown (v/hz) = 0.020
Street flow is on [1] side(s) of the street
Distance from curb to property line = 10.000(Ft.)
Slope from curb to property line (v/hz) = 0.018
Gutter width = 1.500(Ft.)
Gutter hike from flowline = 1.000(In.)
Manning's N in gutter = 0.0150
Manning's N from gutter to grade break = 0.0150
Manning's N from grade break to crown = 0.0150
Estimated mean flow rate at midpoint of street =
                                                    8.321 (CFS)
Depth of flow = 0.412(Ft.), Average velocity = 2.549(Ft/s)
Streetflow hydraulics at midpoint of street travel:
Halfstreet flow width = 17.956(Ft.)
Flow velocity = 2.55 (Ft/s)
Travel time = 1.78 min.
                            TC = 15.67 min.
Adding area flow to street
COMMERCIAL subarea type
Runoff Coefficient = 0.891
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 3) = 84.40
Pervious area fraction = 0.100; Impervious fraction = 0.900
Rainfall intensity = 2.162(In/Hr) for a 100.0 year storm
Subarea runoff =0.732(CFS) for0.380Total runoff =8.665(CFS)Total area =
                   0.732(CFS) for 0.380(Ac.)
                                                 4.260(Ac.)
Street flow at end of street = 8.665(CFS)
Half street flow at end of street = 8.665(CFS)
Depth of flow = 0.418(Ft.), Average velocity = 2.575(Ft/s)
Flow width (from curb towards crown) = 18.235(Ft.)
Process from Point/Station 1.000 to Point/Station 3.000
**** CONFLUENCE OF MAIN STREAMS ****
The following data inside Main Stream is listed:
In Main Stream number: 1
Stream flow area = 4.260 (Ac.)
Runoff from this stream = 8.665(CFS)
Time of concentration = 15.67 min.
Rainfall intensity = 2.162(In/Hr)
Program is now starting with Main Stream No. 2
```

Process from Point/Station 21.000 to Point/Station 3.000 \*\*\*\* INITIAL AREA EVALUATION \*\*\*\* Initial area flow distance = 235.000(Ft.) Top (of initial area) elevation = 1453.220(Ft.) Bottom (of initial area) elevation = 1451.450(Ft.) Difference in elevation = 1.770 (Ft.) Slope = 0.00753 s(percent) = 0.75  $TC = k(0.300) * [(length^3) / (elevation change)]^{0.2}$ Initial area time of concentration = 7.082 min. Rainfall intensity = 3.191(In/Hr) for a 100.0 year storm COMMERCIAL subarea type Runoff Coefficient = 0.894 Decimal fraction soil group A = 0.000Decimal fraction soil group B = 0.000Decimal fraction soil group C = 1.000Decimal fraction soil group D = 0.000RI index for soil (AMC 3) = 84.40Pervious area fraction = 0.100; Impervious fraction = 0.900 Initial subarea runoff = 2.566(CFS) 0.900(Ac.) Total initial stream area = Pervious area fraction = 0.100 Process from Point/Station 21.000 to Point/Station 3.000 \*\*\*\* CONFLUENCE OF MAIN STREAMS \*\*\*\* The following data inside Main Stream is listed: In Main Stream number: 2 Stream flow area = 0.900 (Ac.) Runoff from this stream = 2.566(CFS) Time of concentration = 7.08 min. Rainfall intensity = 3.191(In/Hr) Program is now starting with Main Stream No. 3 Process from Point/Station 31.000 to Point/Station 3.000 \*\*\*\* INITIAL AREA EVALUATION \*\*\*\* Initial area flow distance = 235.000(Ft.) Top (of initial area) elevation = 1453.220(Ft.) Bottom (of initial area) elevation = 1451.450(Ft.) Difference in elevation = 1.770(Ft.) Slope = 0.00753 s(percent) = 0.75  $TC = k(0.300) * [(length^3) / (elevation change)]^{0.2}$ Initial area time of concentration = 7.082 min. Rainfall intensity = 3.191(In/Hr) for a 100.0 year storm COMMERCIAL subarea type Runoff Coefficient = 0.894Decimal fraction soil group A = 0.000Decimal fraction soil group B = 0.000Decimal fraction soil group C = 1.000Decimal fraction soil group D = 0.000RI index for soil (AMC 3) = 84.40 Pervious area fraction = 0.100; Impervious fraction = 0.900 Initial subarea runoff = 2.566(CFS) Total initial stream area = 0.900(Ac.) Pervious area fraction = 0.100

Process from Point/Station 31.000 to Point/Station 3.000 \*\*\*\* CONFLUENCE OF MAIN STREAMS \*\*\*\* The following data inside Main Stream is listed: In Main Stream number: 3 Stream flow area = 0.900(Ac.) Runoff from this stream = 2.566(CFS) Time of concentration = 7.08 min. Rainfall intensity = 3.191(In/Hr) Program is now starting with Main Stream No. 4 Process from Point/Station 41.000 to Point/Station 3.000 \*\*\*\* INITIAL AREA EVALUATION \*\*\*\* Initial area flow distance = 235.000(Ft.) Top (of initial area) elevation = 1453.220(Ft.) Bottom (of initial area) elevation = 1451.450(Ft.) Difference in elevation = 1.770(Ft.) Slope = 0.00753 s(percent) = 0.75  $TC = k(0.300) * [(length^3) / (elevation change)]^{0.2}$ Initial area time of concentration = 7.082 min. Rainfall intensity = 3.191(In/Hr) for a 100.0 year storm COMMERCIAL subarea type Runoff Coefficient = 0.894 Decimal fraction soil group A = 0.000Decimal fraction soil group B = 0.000Decimal fraction soil group C = 1.000Decimal fraction soil group D = 0.000RI index for soil (AMC 3) = 84.40Pervious area fraction = 0.100; Impervious fraction = 0.900 Initial subarea runoff = 2.566(CFS) Total initial stream area = 0.900(Ac.) Pervious area fraction = 0.100 Process from Point/Station 41.000 to Point/Station 3.000 \*\*\*\* CONFLUENCE OF MAIN STREAMS \*\*\*\* The following data inside Main Stream is listed: In Main Stream number: 4 Stream flow area = 0.900 (Ac.) Runoff from this stream = 2.566(CFS) Time of concentration = 7.08 min. Rainfall intensity = 3.191(In/Hr) Program is now starting with Main Stream No. 5 Process from Point/Station 51.000 to Point/Station 3.000 \*\*\*\* INITIAL AREA EVALUATION \*\*\*\* Initial area flow distance = 235.000(Ft.) Top (of initial area) elevation = 1453.220(Ft.) Bottom (of initial area) elevation = 1451.450(Ft.) Difference in elevation = 1.770(Ft.) Slope = 0.00753 s(percent) = 0.75  $TC = k(0.300) * [(length^3) / (elevation change)]^{0.2}$ Initial area time of concentration = 7.082 min. Rainfall intensity = 3.191(In/Hr) for a 100.0 year storm COMMERCIAL subarea type

```
Runoff Coefficient = 0.894
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC 3) = 84.40
Pervious area fraction = 0.100; Impervious fraction = 0.900
Initial subarea runoff = 5.303(CFS)
Total initial stream area =
                          1.860(Ac.)
Pervious area fraction = 0.100
Process from Point/Station 51.000 to Point/Station
                                                        3.000
**** CONFLUENCE OF MAIN STREAMS ****
The following data inside Main Stream is listed:
In Main Stream number: 5
Stream flow area = 1.860(Ac.)
Runoff from this stream = 5.303(CFS)
Time of concentration = 7.08 min.
Rainfall intensity = 3.191(In/Hr)
Summary of stream data:
Stream Flow rate TC
No. (CFS) (min)
                                Rainfall Intensity
                                  (In/Hr)
       8.665 15.67
1
                              2.162
                7.08
2
       2.566
                               3.191
                7.08
3
       2.566
                               3.191
             7.08
7.08
4
       2.566
                               3.191
5
       5.303
                               3.191
Largest stream flow has longer time of concentration
       8.665 + sum of
Qp =
       Qb Ia/Ib
2.566 * 0.678 =
Qb Ia/Ib
2.566 * 0.678 =
                             1.739
        2.566 *
                  0.678 =
                             1.739
                Ia/Ib
       Ob
        2.566 *
                0.678 =
                             1.739
                Ia/Ib
       Qb
        5.303 *
                0.678 =
                             3.593
Qp =
       17.474
Total of 5 main streams to confluence:
Flow rates before confluence point:
                                     2.566 5.303
     8.665 2.566 2.566
Area of streams before confluence:
       4.260 0.900 0.900 0.900 1.860
Results of confluence:
Total flow rate = 17.474 (CFS)
Time of concentration = 15.675 min.
Effective stream area after confluence = 8.820(Ac.)
Process from Point/Station 3.000 to Point/Station 3.000
**** SUBAREA FLOW ADDITION ****
```

COMMERCIAL subarea type

```
Runoff Coefficient = 0.891

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.000

Decimal fraction soil group D = 0.000

RI index for soil (AMC 3) = 84.40

Pervious area fraction = 0.100; Impervious fraction = 0.900

Time of concentration = 15.67 min.

Rainfall intensity = 2.162(In/Hr) for a 100.0 year storm

Subarea runoff = 1.406(CFS) for 0.730(Ac.)

Total runoff = 18.880(CFS) Total area = 9.550(Ac.)

End of computations, total study area = 9.55 (Ac.)

The following figures may

be used for a unit hydrograph study of the same area.

Area averaged pervious area fraction(Ap) = 0.100

Area averaged RI index number = 69.0
```

Appendix C Hydrologic Soils Group and Isohyetal Maps

